



Load Table for Standard Solutions Schöck Isokorb® Type S22

Schöck Isokorb[®] Type S22. Steel-to-steel thermal break connections.

Schock Isokorb® Type S22 is an extremely efficient thermal break solution capable of carrying high bending moment and shear forces in steel-to-steel connections penetrating the building envelope, while eliminating creep concerns commonly associated with pad type connections. It is ideal for engineers and architects who want to improve the thermal performance of the building envelope, eliminate condensation concerns at structural penetrations, and have the peace of mind that comes from working with a company of more than 35 years of experience providing 10+ million structural thermal breaks all around the world.

This document contains load tables for typical steelto-steel connections using Schock Isokorb® Type S22 structural thermal break modules. These load tables can be used to quickly determine the appropriate number of modules required in the connection. For other applications, other beam sections, or other load types (torsion and/or horizontal shear), more detailed analyses should be considered, in collaboration with Schöck, if necessary.

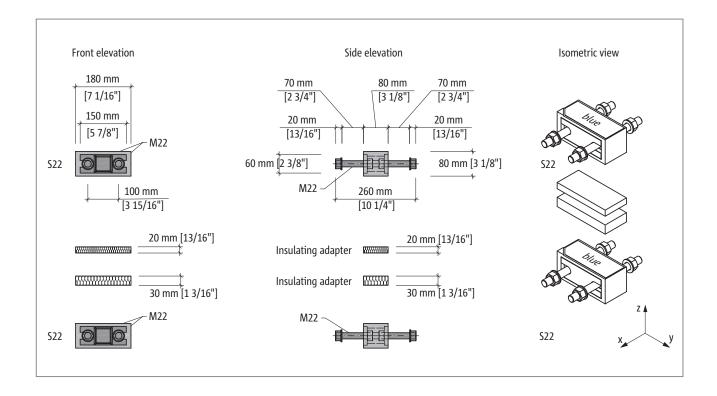
If you have questions, your Schöck Engineering Design team stands by, ready to help.

General comments on load tables:

- All values provided are maximum design ultimate force for the particular design case.
- Loads should be primarily static loads.
- Not for use in the primary load path of the structure (connections typically for cantilever projections)
- Expansion joints may be required, according to the manual.
- The strength values provided apply to the Isokorb connection only. The capacity of the beams entering the connection should be checked independently.
- If there is any doubt, please contact Schöck.

Two Schöck Isokorb® Type S22 combination

Detailed view and coordinate system



Standard solution Schöck Isokorb® Type S22 Imperial units

Beam size			W6×12		W8×10		W10×12		W12×14		W14×22		W16×26		W18×35		
Case ¹			1	2	1	2	1	2	1	2	1	2	1	2	1	2	End-plate Thickness ⁴
Beam flexural strength [kip-ft] F _y = 50 ksi			30.42		32.50		45.42		62.08		120.8		160.0		240.0		THICKNESS
2 modules ou	itside flanges																
	φM _n [kip-ft]		-37.0	-18.5	-44.8	-22.4	-53.3	-26.6	-61.8	-30.9	-69.4	-34.7	-77.9	-38.9	-86.4	-43.2	
	φV _n [kips]		+11.2	+19.3	+11.2	+19.3	+11.2	+19.3	+11.2	+19.3	+11.2	+19.3	+11.2	+19.3	+11.2	+19.3	1 1 /0
	Insulation	30 mm	4		5		8		9		1	0	1	.3	1	4	1 1/8
	spacers	20 mm	:	1	:	2	-	_	1	L	2	2	-	_	1	1	
2 modules wi	ithin flanges																
	φM _n [kip-ft]				-19.8	-9.9	-28.3	-14.1	-36.6	-18.3	-43.3	-21.6	-51.7	-25.8	-59.5	-29.7	3/4
	φV _n [kips]			1.4.2	+11.2	+19.3	+11.2	+19.3	+11.2	+19.3	+11.2	+19.3	+11.2	+19.3	+11.2	+19.3	
	Insulation	30 mm	N/A ²		2		3		4		6		7		8		3/4
	spacers	20 mm															
4 modules ar	ound flanges																
	φM _n [kip-ft]						N/A ³		-83.0	-41.5	-96.3	-48.1	-112.1	-56.0	-127.5	-63.7	
	φV _n [kips]		N/A ²			/ 4 3			+29.7	+38.6	+28.9	+38.6	+28.2	+38.6	+27.7	+38.6	
	Insulation	30 mm	N/	A-	N/A ³		IN/	'A'	3		6		5		8		1
	spacers	20 mm							2	2	-	-	4		2		
8 modules ar	ound flanges																
9_C 9_C 9_C 9_C	φM _n [kip-ft]	φM _n [kip-ft] φV _n [kips] Insulation 30 mm									-192.7	-96.3	-224.2	-112.1	-255.0	-127.5	
	φV _n [kips]			N/A ²		/ 4 3	, p. 1	/ 4 3	N/A³		+57.9	+77.3	+56.5	+77.3	+55.4	+77.3	
0_0_0_0_0	Insulation					/A³	N/	/A³			12		10		1	6	1
0_0_0_0	spacers	20 mm	1								_		8		4		1

Beam size			HSS 4>	4×1/2	HSS 6×6×1/2		HSS 8>	×8×1/2	HSS 10	HSS 10×10×1/2		×12×1/2		
Case ¹			1	2	1	2	1	2	1	2	1	2	End-plate Thickness⁴	
Beam flexura F _y = 50 ksi	Beam flexural strength [kip-ft] Fy = 50 ksi			24.9		67.1		130.0		213.0		317.0		
2 modules outside flanges														
	φM _n [kip-ft]		-28.5	-14.2	-36.9	-18.4	-45.4	-22.7	-53.8	-26.9	-62.3	-31.1		
	φV _n [kips]		+11.2	+19.3	+11.2	+19.3	+11.2	+19.3	+11.2	+19.3	+11.2	+19.3	11/0	
	Insulation	30 mm	3	3		1	5		8		5		1 1/8	
	spacers 20 mm		_		1		2		-		2			

¹ Case 1 represents values for highest moment capacity.

¹ Case 1 represents values for highest moment capacity.
Case 2 represents values for half of highest moment capacity (shear value depends on moment value).
² This configuration is not feasible due to lack of space for Isokorb[®] modules within the beam depth.
³ This configuration is not necessary for structural steel of ≤50 ksi. The respective beam strength may be matched or exceeded by the simpler and more economical Isokorb[®] configuration shown on the table. Such configurations may be considered for higher strength beams, if necessary.
⁴ The end-plate thickness is provided for estimation and constructibility evaluation purposes only. Steel of ≥50ksi has been assumed. The Structural Engineer of Record will need to evaluate these values for the specifics of the project. The use of stiffeners might result in a lower thickness value.

Standard solution Schöck Isokorb® Type S22 Metric units

Beam size			W15	0×18	W20	0×15	W250×18		W310×21		W360×33		W410×39		W460×52		
Case ¹			1	2	1	2	1	2	1	2	1	2	1	2	1	2	End-plate Thickness⁴
Beam flexural strength [kNm] Fy = 345MPa		41.3		44.1		61.8		84.2		163		217		326		mickness	
2 modules o	outside flang	es															
	φM _n [kNm]	-50.2	-25.0	-60.8	-30.4	-72.3	-36.1	-83.8	-41.9	-94.2	-47.1	-105.7	-52.8	-117.2	-58.6	
	φV _n [kN]		+50.0	+86.0	+50.0	+86.0	+50.0	+86.0	+50.0	+86.0	+50.0	+86.0	+50.0	+86.0	+50.0	+86.0	30mm
	Insulation	30 mm	4	1	5	5	8	3	9	Ð	1	0	1	3	1	4	John
	spacers	20 mm	1	L	:	2	-	-	:	1	2	2	-	-	1	L	
2 modules within flanges																	
	φM _n [kNm]	N/A ²		-26.9	-13.5	-38.4	-19,2	-49.7	-24.8	-58.8	-29.4	-70.2	-35.1	-80.7	-40.3	20mm
	φV _n [kN]				+50.0	+86.0	+50.0	+86.0	+50.0	+86.0	+50.0	+86.0	+50.0	+86.0	+86.0 +50.0 +86.0	+86.0	
	Insulation 30	30 mm			-		3		4		6		7		8	3	2011111
	spacers	acers 20 mm				2		-		1		_		1		2	
4 modules a	4 modules around flanges																
	φM _n [kNm]							-112.6	-56.3	-130.6	-65.3	-152.0	-76.0	-172.8	-86.4	
	φV _n [kN]		N/A²		N/A³		51/63		+132.4	+172.0	+128.8	+172.0	+125.7	+172.0	+123.3	+172.0	25mm
ØØ	Insulation	30 mm					11/	N/A ³		3		6		5		3	2511111
	spacers	20 mm							2		-		4		2		
8 modules a	round flang	es															
	φM _n [kNm]									-261.3	-130.6	-303.9	-152.0	-345.7	-172.8	
	φV _n [kN]		N/	Λ2	N	′A³	NI	۸3	N/A ³		+257.6	+344.0	+251.5	+344.0	+246.6	+344.0	25mm
9-0 9-0	Insulation	30 mm	11/	А		A	N/A ³		11/	A	12		10		1	6	2511111
900 900	spacers	20 mm									-		8		4		

Beam size			HSS 101.6×101.6×12×7		HSS 152.4×152.4×12.7		HSS 203.2×203.2×12.7		HSS 254.0×254.0×12.7		HSS 304.8×304.8×12.7			
Case ¹			1	2	1	2	1	2	1	2	1	2	End-plate Thickness⁴	
Beam flexural strength [kNm] F _y = 345MPa			33	8.7	91.1		176		289		431			
2 modules o	2 modules outside flanges													
	φM _n [kNm]		-38.6	-19.3	-50.1	-25.0	-61.5	-30.7	-73.0	-36.5	-84.4	-42.2		
	φV _n [kN]		+50.0	+86.0	+50.0	+86.0	+50.0	+86.0	+50.0	+86.0	+50.0	+86.0	20,000,000	
	Insulation	30 mm	3	3	4	1	<u>.</u>	5		8		5	30mm	
	spacers	20 mm	-	_	:	1		2	-	_		2		

¹ Case 1 represents values for highest moment capacity. Case 2 represents values for half of highest moment capacity (shear value depends on moment value).
² This configuration is not feasible due to lack of space for Isokorb[®] modules within the beam depth.
³ This configuration is not necessary for structural steel of 345MPa. The respective beam strength may be matched or exceeded by the simpler and

more economical Isokorb[®] configuration shown on the table. Such configurations may be considered for higher strength beams, if necessary. ⁴ The end-plate thickness is provided for estimation and constructibility evaluation purposes only. Steel of 345MPa has been assumed. The Structural Engineer of Record will need to evaluate these values for the specifics of the project. The use of stiffeners might result in a lower thickness value.

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