Schöck Isokorb® XT type D



Schöck Isokorb® XT type D

Load-bearing thermal insulation element for continuous flooring. The element transfers moments and shear forces.



Element arrangement | Installation cross sections



Fig. 244: Schöck Isokorb® XT type D, Q-Z: One-way spanning







Fig. 246: Schöck Isokorb® XT type D: One-way spanning



Element arrangement

- With connection across the corner with Schöck Isokorb® XT type D, a type D-CV50 (2nd position) is required in one axial direction Therefore a minimum slab thickness of 200 mm.
- The Schöck Isokorb® transmits moments vertically to the insulation joint, it transmit no moments parallel to the insulation joint. Therefore it is not suitable for employment within point supported floor bays or in balconies with 4 columns.

XT type D

Product selection | Type designations | Special designs

Schöck Isokorb® XT type D variants

The configuration of the Schöck Isokorb® XT type D can vary as follows:

- Main load-bearing level:
- MM1 to MM5 Secondary load-bearing level:
- VV1 to VV5 Fire resistance class:

REI120 (standard): Top and bottom fire protection projecting by 10mm on both sides

- Concrete cover of the tension bars:
 CV35: Top CV = 35 mm, bottom CV = 30 mm
 CV50: Top CV = 50 mm, bottom CV = 50 mm
- Insulating element thickness:
- X120 = 120 mm
- Isokorb[®] height:

 $H = H_{min}$ to 250 mm (H_{min} depends on the concrete cover and shear force load-bearing level, see page 160)

- Generation:
- 5.0

Type designations in planning documents



Special designs

Please contact the design support department if you have connections that are not possible with the standard product variants shown in this information (contact details on page 3).

In accordance with approval heights up to 500 mm are possible.

Cale Sale Ia	alaank® VT tu	ma D		MM1		MM2			
SCHOCK IS	OKOLD® XI TY	peD	VV1	VV2	VV3	VV1	VV2	VV3	
Design values	Concre CV	te cover [mm]	Concrete strength class ≥ C25/30						
WILII	CV35	CV50	m _{Rd,y} [kNm/m]						
	160		±14.7	±13.8	-	±17.9	-	-	
		200	±15.5	±14.7	-	±19.0	-	-	
	170		±16.4	±15.5	±13.3	±20.1	±17.9	-	
		210	±17.3	±16.3	±14.0	±21.1	±18.8	-	
	180		±18.2	±17.1	±14.7	±22.2	±19.8	±16.7	
		220	±19.1	±18.0	±15.4	±23.3	±20.8	±17.5	
	190		±20.0	±18.8	±16.2	±24.4	±21.7	±18.3	
lsokorb® height		230	±20.8	±19.6	±16.9	±25.4	±22.7	±19.1	
H [mm]	200		±21.7	±20.5	±17.6	±26.5	±23.6	±19.9	
		240	±22.6	±21.3	±18.3	±27.6	±24.6	±20.7	
	210		±23.5	±22.1	±19.0	±28.7	±25.6	±21.5	
		250	±24.4	±23.0	±19.7	±29.8	±26.5	±22.3	
	220		±25.2	±23.8	±20.4	±30.8	±27.5	±23.2	
	230		±27.0	±25.5	±21.9	±33.0	±29.4	±24.8	
	240		±28.8	±27.1	±23.3	±35.2	±31.3	±26.4	
	250		±30.5	±28.8	±24.7	±37.3	±33.2	±28.0	
					V _{Rd,z} [k	(N/m]			
Secondary load-b	earing level	VV1 – VV3	±28.2	±42.3	±75.2	±42.3	±75.2	±117.5	

Cahäak kakayh® VI tuna D	MM1			MM2			
	VV1	VV2	VV3	VV1	VV2	VV3	
Discoment with			Isokorb® le	ngth [mm]			
	1000						
Tension bars/compression members	2 × 4 Ø 12			2 × 5 Ø 12			
Shear force bars	2 x 4 Ø 6	2 x 6 Ø 6	2 x 6 Ø 8	2 x 6 Ø 6	2 x 6 Ø 8	2 x 6 Ø 10	
H _{min} with CV35 [mm]	160	160	170	160	170	180	
H _{min} with CV50 [mm]	200	200	210	200	210	220	



Fig. 248: Schöck Isokorb® XT type D: Static system

Schöck Isokorh® XT tune D		no D	ММЗ					
SCHOCK IS	OKORD® X I TY	ре и	VV1	VV2	VV3	VV4	VV5	
Design values	Concret CV [e cover mm]	Concrete strength class ≥ C25/30					
WILII	CV35	CV50	m _{Rd,y} [kNm/m]					
	160		±26.1	-	-	-	-	
		200	±27.6	-	-	-	-	
	170		±29.2	±27.0	-	-	-	
		210	±30.8	±28.5	-	-	-	
	180		±32.3	±29.9	±26.8	±23.9	-	
		220	±33.9	±31.4	±28.1	±25.1	-	
	190		±35.5	±32.8	±29.4	±26.3	±20.7	
Isokorb® height		230	±37.1	±34.3	±30.7	±27.4	±21.6	
H [mm]	200		±38.6	±35.7	±32.0	±28.6	±22.5	
		240	±40.2	±37.2	±33.3	±29.7	±23.4	
	210		±41.8	±38.6	±34.6	±30.9	±24.4	
		250	±43.3	±40.1	±35.9	±32.1	±25.3	
	220		±44.9	±41.5	±37.2	±33.2	±26.2	
	230		±48.0	±44.4	±39.8	±35.5	±28.0	
	240		±51.2	±47.4	±42.4	±37.9	±29.8	
	250		±54.3	±50.3	±45.0	±40.2	±31.7	
					v _{Rd,z} [kN/m]			
Secondary load-b	earing level	VV1 – VV5	±42.3	±75.2	±117.5	±156.7	±225.6	

Cabial Icalianh® VT tura D	ММЗ						
	VV1	VV2	VV3	VV4	VV5		
Discoment with	Isokorb® length [mm]						
	1000						
Tension bars/compression members	2 × 7 Ø 12						
Shear force bars	2 x 6 Ø 6	2 x 6 Ø 8	2 x 6 Ø 10	2 x 8 Ø 10	2 x 8 Ø 12		
H _{min} with CV35 [mm]	160	170	180	180	190		
H _{min} with CV50 [mm]	200	210	220	220	230		

Schöekte	Schöck Isokorh® XT type D		MM4					
SCHOCK IS	OKOTD [®] XI ty	pe D	VV1	VV2	VV3	VV4	VV5	
Design values	Concret CV [e cover mm]	Concrete strength class ≥ C25/30					
with	CV35	CV50	m _{Rd,y} [kNm/m]					
	160		±38.3	-	-	-	-	
		200	±40.6	-	-	-	-	
	170		±42.9	±40.7	-	-	-	
		210	±45.2	±42.9	-	-	-	
	180		±47.5	±45.1	±42.0	±39.1	-	
		220	±49.8	±47.3	±44.0	±41.0	-	
	190		±52.2	±49.5	±46.1	±42.9	±37.4	
Isokorb® height		230	±54.5	±51.7	±48.1	±44.8	±39.0	
H [mm]	200		±56.8	±53.9	±50.2	±46.7	±40.7	
		240	±59.1	±56.1	±52.2	±48.6	±42.3	
	210		±61.4	±58.3	±54.2	±50.5	±44.0	
		250	±63.7	±60.4	±56.3	±52.4	±45.6	
	220		±66.0	±62.6	±58.3	±54.3	±47.3	
	230		±70.6	±67.0	±62.4	±58.1	±50.6	
	240		±75.2	±71.4	±66.5	±61.9	±53.9	
	250		±79.8	±75.8	±70.6	±65.7	±57.2	
					v _{Rd,z} [kN/m]			
Secondary load-b	earing level	VV1 – VV5	±42.3	±75.2	±117.5	±156.7	±225.6	

Sehäck leekerb® VT ture D	MM4						
	VV1	VV2	VV3	VV4	VV5		
Discoment with	Isokorb® length [mm]						
	1000						
Tension bars/compression members	2 × 10 Ø 12						
Shear force bars	2 x 6 Ø 6	2 x 6 Ø 8	2 x 6 Ø 10	2 x 8 Ø 10	2 x 8 Ø 12		
H _{min} with CV35 [mm]	160	170	180	180	190		
H _{min} with CV50 [mm]	200	210	220	220	230		

Schöck Isakarh® XT tung D		no D	MM5					
SCHOCK IS	OKORD® X I TY	ре и	VV1	VV2	VV3	VV4	VV5	
Design values	Concret CV [te cover mm]	Concrete strength class ≥ C25/30					
with	CV35	CV50	m _{Rd,y} [kNm/m]					
	160		±46.5	-	-	-	-	
		200	±49.3	-	-	-	-	
	170		±52.1	±49.9	-	-	-	
		210	±54.9	±52.6	-	-	-	
	180		±57.7	±55.2	±52.1	±49.3	-	
		220	±60.5	±57.9	±54.7	±51.6	-	
	190		±63.3	±60.6	±57.2	±54.0	±48.5	
Isokorb® height		230	±66.1	±63.3	±59.7	±56.4	±50.6	
H [mm]	200		±68.9	±66.0	±62.3	±58.8	±52.8	
		240	±71.7	±68.7	±64.8	±61.2	±54.9	
	210		±74.5	±71.3	±67.3	±63.6	±57.1	
		250	±77.3	±74.0	±69.8	±66.0	±59.2	
	220		±80.1	±76.7	±72.4	±68.4	±61.3	
	230		±85.7	±82.1	±77.4	±73.2	±65.6	
	240		±91.3	±87.4	±82.5	±77.9	±69.9	
	250		±96.9	±92.8	±87.6	±82.7	±74.2	
					v _{Rd,z} [kN/m]			
Secondary load-b	earing level	VV1 – VV5	±42.3	±75.2	±117.5	±156.7	±225.6	

Schöck Icokorh® VT tuno D	MM5						
	VV1	VV2	VV3	VV4	VV5		
Discoment with			sokorb® length [mn	ו]			
	1000						
Tension bars/compression members			2 × 12 Ø 12				
Shear force bars	2 x 6 Ø 6	2 x 6 Ø 8	2 x 6 Ø 10	2 x 8 Ø 10	2 x 8 Ø 12		
H _{min} with CV35 [mm]	160	170	180	180	190		
H _{min} with CV50 [mm]	200	210	220	220	230		

Notes on design

- With different concrete strength classes (e.g. balcony C32/40, inner slab C25/30) basically the weaker concrete is relevant for the design of the Schöck Isokorb[®].
- The indicative minimum concrete strength class of the external structural component is C32/40.
- A static verification is to be provided for the adjacent reinforced concrete structural component on both sides of the Schöck Isokorb[®].
- The shear force loading of the slabs in the area of the insulation joint is to be limited to $V_{Rd, max}$, whereby $V_{Rd, max}$, acc. to BS EN 1992-1-1 (EC2), Exp. (6.9) is determined for θ = 45 ° and α = 90 ° (slab load-bearing capacity).
- The Schöck Isokorb® XT type D transmits only bending moments perpendicular to the insulation slab. The Schöck Isokorb® transmits no torsion moments. Therefore the arrangement of a Schöck Isokorb® XT type D is not sensible in a punctually supported slab without downstand beams.

XT type D

Deflection/Camber

Deflection

The deflection factors given in the table (tan α [%]) result alone from the deflection of the Schöck Isokorb[®] under 100% steel utilisation. They serve for the estimation of the required camber. The total arithmetic camber of the balcony slab formwork results from the calculation according to BS EN 1992-1-1 (EC2) and BS EN 1992-1-1/NA plus the deflection from Schöck Isokorb[®]. The camber of the balcony slab formwork to be given by the structural engineer/designer in the implementation plans (Basis: Calculated total deflection from cantilever slab + floor rotation angle + Schöck Isokorb[®]) should be so rounded that the scheduled drainage direction is maintained (round up: with drainage to the building facade, round down: with drainage towards the cantilever slab end).

Deflection (p) as a result of Schöck Isokorb®

	р	= tan $\alpha \cdot l_{k} \cdot (m_{pd} / m_{Rd}) \cdot 10 [mm]$
Factors to be applied		
	$\tan \alpha$	= apply value from table
	l _k	= cantilever length [m]
	\mathbf{m}_{pd}	= relevant bending moment [kNm/m] in the ultimate limit state for the determination of the p [mm] from Schöck Isokorb [®] .
		The load combination to be applied for the deflection is determined by the structural engineer.
		(Recommendation: Load combination for the determination of the camber p : determine $q+q/2$, m_{pd} in the ultimate limit state)
	m _{Rd}	= maximum design moment [kNm/m] of the Schöck Isokorb®



Fig. 249: Schöck Isokorb® XT type D: Static system

Schöck Isokorb® XT ty	pe D	MM1–MM5				
Deflection factor for		CV35	CV50			
Deflection factor to	ſ	tan α [%]				
	160	1.2	-			
	170	1.0	-			
	180	0.9	-			
	190	0.8	-			
leakarh® haight [] [mm]	200	0.7	1.1			
Isokord° neight A [mm]	210	0.6	1.0			
	220	0.6	0.8			
	230	0.6	0.7			
	240	0.5	0.7			
	250	0.5	0.6			

Expansion joint spacing

Maximum expansion joint spacing

If the length of the structural component length exceeds the maximum expansion joint spacing e, then the expansion joints must be integrated into the external concrete components at right angles to the insulating layer in order to limit the effect as a result of temperature changes. With fixed points such as, for example, balcony corners or with the employment of the Schöck Isokorb[®] XT types H, half the maximum expansion joint spacing e/2 applies.

Schöck Isokorb® XT type D		MM1 MM2–MM5 MM2 VV1–VV3 VV1–VV2 VV3			MM3–MM5 VV3–VV4	MM3–MM5 VV5
Maximum expansion joint sp	e [m]					
Insulating element thick- ness [mm]	120	19.8	19.8	19.5	19.5	17.7

Edge distances

The Schöck Isokorb® must be so arranged at the expansion joint that the following conditions are met:

- For the centre distance of the tension bars from the free edge or from the expansion joint: $e_R \ge 50$ mm and $e_R \le 150$ mm applies.
- For the centre distance of the compression bars from the free edge or the expansion joint the following applies: e_R ≥ 50 mm and e_R ≤ 150 mm.
- For the centre distance of the shear force bars from the free edge or from the expansion joint the following applies: $e_R \ge 100$ mm and $e_R \le 150$ mm.

Product description



Fig. 250: Schöck Isokorb® XT type D with CV35: Product section



Fig. 252: Schöck Isokorb® XT type D-MM3-VV1: Plan view



Fig. 251: Schöck Isokorb® XT type D with CV50: Product section



Fig. 253: Schöck Isokorb® XT type D-MM3-VV5: Layout

Product information

Download further product plan views and cross-sections at cad.schoeck.co.uk

XT type D

On-site reinforcement



Fig. 254: Schöck Isokorb® XT type D: On-site reinforcement

II Information about on-site reinforcement

- The rules according to BS EN 1992-1-1 (EC2) and BS EN 1992-1-1/NA apply for the determination of the overlap length. A reduction of the required overlap length with m_{Ed}/m_{Rd} is permitted. For the overlap (l) with the Schöck Isokorb[®] for the XT type D a length of the tension bars of 605 can be brought to account.
- An edge and suspension reinforcement (Pos. 3) is to be arranged on both sides of the Schöck Isokorb[®] XT type D. Details in the table apply for Schöck Isokorb[®] with a loading of 100% of the maximum design internal forces with 25/30.

On-site reinforcement

Recommendation for the on-site connection reinforcement

Information on the on-site reinforcement for Schöck Isokorb[®] with a loading of 100 % of the maximum design moment and the shear force with C25/30. The required reinforcement cross-section depends on the bar diameter of the steel bar or wire-mesh reinforcement – see type approval.

Schöck Isokorb®	VT tuno D		MM1			MM2		
SCHOCK ISOKOID®	л туре о		VV1	VV2	VV3	VV1	VV2	VV3
On-site	CV35	CV50	Floor (XC1) concrete strength class ≥ C25/30					
reinforcement	Heigh	t [mm]	Balcony (XC4) concrete strength class ≥ C25/30					
Lap reinforcement dependent on bar diameter (necessary for negative moment)								
Pos. 1 with Ø8 [mm²/m]			491	511	467	624	580	565
Pos. 1 with Ø10 [mm²/m]		524 560 532 673			673	646	589	
Pos. 1 with Ø12 [mm²/m]			595 643 620			768	745	690
Steel bars along the insul	ation joint							
Pos. 2					2•2	• H8		
Vertical reinforcement								
Pos. 3 [mm ² /m]	160-180	200–210			11	13		
Pos. 3 [mm ² /m]	190–250	220-250	113	113	173	113	173	270
Lap reinforcement depend	dent on ba	r diameter	(necessary for	positive momer	nt)			
Pos. 4 with H8 [mm ² /m]			491	511	467	624	580	565
Pos. 4 with H10 [mm ² /m]			524 560 532 673 646 5					589
Pos. 4 with H12 [mm ² /m]			595	643	620	768	745	690

Cabiak Isakash® VI tuna D			ММЗ					
SCHOCK ISOKOID®	хт туре о		VV1	VV2	VV3	VV4 ASS ≥ C25/30	VV5	
On-site	CV35	CV50	Floor (XC1) concrete strength class ≥ C25/30					
reinforcement	Height [mm]		Balcony (XC4) concrete strength class ≥ C25/30					
Lap reinforcement depen	dent on ba	r diameter	(necessary for neg	gative moment)				
Pos. 1 with Ø8 [mm ² /m]			850	806	792	792	792	
Pos. 1 with Ø10 [mm²/m]			899	872	816	823	792	
Pos. 1 with Ø12 [mm²/m]			1018	995	940	962	797	
Steel bars along the insul	ation joint							
Pos. 2			2 • 2 • H8					
Vertical reinforcement								
Pos. 3 [mm²/m]	160-180	200–210	113	113	113	113	113	
	190–250	220–250	113	173	270	360	519	
Lap reinforcement depen	dent on ba	r diameter	(necessary for pos	sitive moment)				
Pos. 4 with H8 [mm²/m]			850	806	792	792	792	
Pos. 4 with H10 [mm ² /m]			899	872	816	823	792	
Pos. 4 with H12 [mm ² /m]			1018	995	940	962	797	

children WT town D			MM4					
SCHOCK ISOKORD® XI TYPE D		VV1	VV2	VV3	VV4	VV5		
On-site reinforcement	CV35	CV50	Floor (XC1) concrete strength class ≥ C25/30					
	Height [mm]		Balcony (XC4) concrete strength class ≥ C25/30					
Lap reinforcement dependent on bar diameter (necessary for negative moment)								
Pos. 1 with Ø8 [mm²/m]			1189	1146	1131	1131	1131	
Pos. 1 with Ø10 [mm²/m]			1239	1211	1155	1163	1131	
Pos. 1 with Ø12 [mm²/m]			1393	1370	1315	1337	1172	
Steel bars along the insulation joint								
Pos. 2			2 · 2 · H8					
Vertical reinforcement								
Pos. 3 [mm²/m]	160–180	200-210	113	113	113	113	113	
	190–250	220-250	113	173	270	360	519	
Lap reinforcement dependent on bar diameter (necessary for positive moment)								
Pos. 4 with H8 [mm ² /m]			1189	1146	1131	1131	1131	
Pos. 4 with H10 [mm ² /m]			1239	1211	1155	1163	1131	
Pos. 4 with H12 [mm ² /m]			1393	1370	1315	1337	1172	

Schöck Isokorb® XT type D		MM5						
		VV1	VV2	VV3	VV4	VV5		
On-site	CV35	CV50	Floor (XC1) concrete strength class ≥ C25/30					
reinforcement	Height [mm]		Balcony (XC4) concrete strength class ≥ C25/30					
Lap reinforcement depen	dent on ba	ır diameter	(necessary for neg	gative moment)				
Pos. 1 with Ø8 [mm ² /m]			1416	1372	1357	1357	1357	
Pos. 1 with Ø10 [mm²/m]	0 [mm ² /m] 1465 1437 1381 1389			1357				
Pos. 1 with Ø12 [mm ² /m]			1643	1620	1566	1587	1422	
Steel bars along the insul	ation joint							
Pos. 2			2 · 2 · H8					
Vertical reinforcement								
Pos. 3 [mm ² /m]	160-180	200–210	113	113	135	120	173	
	190–250	220-250	113	173	270	360	519	
Lap reinforcement depen	dent on ba	ır diameter	(necessary for pos	sitive moment)				
Pos. 4 with H8 [mm ² /m]			1416	1372	1357	1357	1357	
Pos. 4 with H10 [mm ² /m]			1465	1437	1381	1389	1357	
Pos. 4 with H12 [mm ² /m] 1643 1620 1566 1587				1422				

IInstallation instructions

The current installation instruction can be found online under: www.schoeck.com/view/6424

Check list

- Have the loads on the Schöck Isokorb[®] connection been specified at design level?
- Has the cantilevered system length or the system support width been taken as a basis?
- Are the maximum allowable expansion joint spacings taken into account?
- □ With the selection of the design table is the relevant concrete cover taken into account?
- Are the requirements with regard to fire protection explained and is the appropriate addendum entered in the Isokorb[®] type description in the implementation plans?
- □ Is the minimum slab thickness (≥ 200 mm) and the required concrete cover (-CV50) taken into account with connection over a corner using Schöck Isokorb[®] XT type D?
- □ With XT type D in conjunction with prefabricated floors is the required block-out (width \geq 650 mm from insulating element) drawn into the implementation plans and is the on-site reinforcement adjusted?
- □ With 2- or 3-sided support is a Schöck Isokorb[®] selected for a connection free of constraint selected (possibly XT type Q-Z, XT type Q-PZ)?
- Have the requirements for on-site reinforcement of connections been defined in each case?